

### **RPUG 2018 CONFERENCE - SOUTH DAKOTA** 30 Years On The Road To Progressively Better Data

Rapid City September 18-21

# High Resolution Multi-Lane Road Surface Mapping using 3D Laser Profilers

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30 years on the Road To Progressively Better Data

# The importance of road smoothness

- Very bumpy roads:
  - 30-40% increase of wear of road
  - Vertical acceleration increases dynamic load impact of traffic
  - Self destruction of bumpy road surfaces
- Driving comfort
- Fixed depth milling operations do NOT improve the longitudinal road profile



# **Road Rehabilitation**



# **Road Rehabilitation**



# LDTM – Surveyor grade Terrain Mapping





# **Pavemetrics LDTM solution**

#### 1. LCMS system

- 2 Laser profilers (4 meters field of view)
- 2 Inertial Measurement Units (IMU)
- Distance Measuring Instrument
- (DMI)



- Optical encoder (DMI)
- Inertial Measurement Unit (IMU)
- GNSS





Asphalt Concrete Gravel

# **3D Road Profile Before Dynamic Corrections**



# **3D Road Profile After Dynamic Corrections**

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Equipment Calibration

#### □ Survey

#### Processing

- Navigation solution
- Controls Points
- Stitch lanes

#### Data Exportation

# **LDTM Calibration**



- 1 Scan of the calibration validation object
- 2 Stop and GO

3 – Measurement of the position of the sensors

Done only once during sensors installation







LDTM Calibration

# **Ground Control Points Survey**

#### **Ground Control Points**

- Surveyed using a robotic total station
- One point every 300 meters on road surface or shoulder
- Converted in UTM

LDTM SURVEY

Imported in LCMS-PV3D software

Data Processing





# **Control and Alignment Points**

#### LCMSPV3D ALIGNMENT POINTS CREATION



Green dot – Ground Control Point Orange dot – Alignment point

Conclusion: The entire surface is shifted down and right





Run 3

# **Tie Points Creation -Automatic mode**

# PV3D - Finds and select common features in overlapping areas



Run 2





# **Tie Points Creation**



# Shift Between Runs Before Processing Results (example)



**Pavemetrics** 

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# No more shift between runs after processing is applied

**Pavemetrics** 

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![](_page_16_Picture_1.jpeg)

![](_page_17_Picture_0.jpeg)

#### **Stitching Runs (before processing)**

![](_page_17_Figure_2.jpeg)

![](_page_17_Figure_3.jpeg)

![](_page_18_Picture_0.jpeg)

#### **Stitching Runs (after processing)**

![](_page_18_Figure_2.jpeg)

![](_page_18_Figure_3.jpeg)

# **Final surface**

![](_page_19_Figure_2.jpeg)

# LAS file (10cm x 10cm)

![](_page_20_Picture_1.jpeg)

![](_page_21_Picture_0.jpeg)

#### Multiple runs - Average error compare to GT

![](_page_21_Figure_2.jpeg)

#### ■X ■Y ■Z

Accuracy compare to GT (Avg. in mm): X: 5.0 Y: 4.0 Z: 2.5 Repeatability compare to first scan (mm)\*: X: 3.0 Y: 5.0 Z: 2.0

## Runway 08-26, Montmagny, QC

![](_page_22_Figure_1.jpeg)

# Runway 08-26, Montmagny, QC

![](_page_23_Picture_1.jpeg)

![](_page_24_Picture_0.jpeg)

![](_page_24_Picture_1.jpeg)

![](_page_25_Picture_0.jpeg)

# **Survey Specifications**

Total length of runway: 900 meters

Total width of runway: 30 meters

6 surveyed control points were used (3 on each end of the runway)

44 surveyed test points were distributed over the entire runway surface.

**Total survey time 15 minutes** 

Total survey length 12km

Number of scans used to cover runway surface: 11\* (total in both directions)

\* A large number of scans were used to insure overlap between runs

# Survey (11 runs with approx. 1m overlap between runs)

![](_page_26_Figure_1.jpeg)

![](_page_26_Figure_2.jpeg)

![](_page_27_Picture_0.jpeg)

![](_page_27_Figure_1.jpeg)

349 tie points were automatically generated and used to automatically stitch the 11 runs

# **Tie Points**

![](_page_28_Picture_2.jpeg)

![](_page_29_Picture_0.jpeg)

6 surveyed control points (located on the ends of the runway) were used to align the Pavemetrics data to the traditional survey

# **Control Points (manual)**

![](_page_30_Picture_2.jpeg)

Green: Position of control point before alignment. Orange: Alignment point

![](_page_30_Picture_4.jpeg)

Green: Position of control point after alignment

![](_page_31_Picture_0.jpeg)

900 metres

30 m 🕻

# Runway 08-26 - result

![](_page_32_Picture_1.jpeg)

![](_page_33_Picture_0.jpeg)

# Runway 08-26 - result

![](_page_33_Picture_2.jpeg)

![](_page_34_Picture_0.jpeg)

# **Accuracy Analysis**

The accuracy of the LDTM solution was evaluated using 44 survey targets which were painted on the runway

Due to the shape of the painted targets X, Y coordinates were difficult to match, however elevation data (Z) was excellent

![](_page_34_Picture_4.jpeg)

What we wanted!

![](_page_34_Picture_6.jpeg)

What we got!

# Survey Target Locations Overlaid on Pavemetrics' Data

![](_page_35_Picture_1.jpeg)

![](_page_35_Figure_2.jpeg)

Surveyed test points used to evaluate the accuracy of the LDTM solution

![](_page_36_Picture_0.jpeg)

# Accuracy analysis

#### **RTK Base GNSS used for the post-processing**

• Base station was in La Pocatière, QC which is 62 km from the site

#### **Survey time was only 15 minutes**

Data preparation and processing time took 3 hours

The entire runway LDTM surface data was then compared to the 44 surveyed points to evaluate the overall errors

![](_page_37_Picture_0.jpeg)

Accuracy versus survey results:

Average RMS Error [X; Y; Z]=[0.016; 0.011; 0.003]m

As expected X, Y accuracy was lower because of shape of painted survey markers.

**Elevation accuracy was an amazing 3mm overall!** 

![](_page_38_Picture_0.jpeg)

# LAS file output (10cm x 10cm)

![](_page_38_Picture_2.jpeg)

# **Questions**?